



A STUDY ON EFFECT OF AGGREGATE GRADATION AND SIZE ON COLD BITUMIN OUS MIX

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Abstract :

In India, about 90% of the total road network including airfield pavement is of flexible pavement, which are constructed using hot mix technology. Hot mix technology has numerous drawbacks. Some of them are liberation of greenhouse gases, high consumption of energy, health hazards to construction labours, etc. Hence, adoption of alternative technologies such as cold mix technology is needed to reduce the drawbacks of hot mix technology.

In this study two aggregate gradations are taken based on the Nominal Maximum Particle Size (NMPS): NMPS 13.2mm and NMPS 19mm from Ministry of Road Transport and Highways (MoRTH) specification. Further, two more aggregate gradations were formulated for each NMPS based on modified Fuller and Thompson maximum density gradation and Cooper's equation with 4% filler. The gradation of MoRTH gradation, modified Fuller and Thompson equation and Cooper's equation for NMPS 13.2mm are designated as M13.2, MFT13.2, and C13.2 respectively and for NMPS 19mm gradations as M19, MFT19 and C19 respectively. Medium setting cationic type bitumen emulsion was used for this study. The study methodology includes characterization of aggregates and emulsion, determination of pre-wetting water content, volumetric analysis of mixes by Marshall Stability test, performance evaluation by Retained Marshall Stability (RMS) test. A parameter called gradation ratio is correlated with strength and performance parameters of the mix to predict later from aggregate gradation curve alone.

Keywords - NMPS, MoRTH, prewetting water content, Marshall Stability test, Retained Marshall Stability.

1. INTRODUCTION

In the emulsion-based cold mix technology, the addition of pre-wetting water to the aggregate, thereafter addition of emulsion

to it, production of the mix, laying and compaction, all processes are done at the room temperature (23°C to 25°C). In addition to this, field trials have proved that cold mix can

be easily produced by using hot mix plant and laid in using similar techniques. It is also a labour friendly technology. It can produce manually for small-scale jobs like patchwork, potholes, and repairs. It is particularly suited for the construction of roads in topographical and weather constraints, remote and isolated areas. Cold bituminous mixture (CBM) is composed of approximately 95% by weight or 80% by volume with mineral aggregate. Therefore, it is essential to see how aggregate gradation affects the performance of cold bituminous mixes. Aggregates form the skeleton of the bituminous mixture and are responsible for the load carrying capacity of pavement. Aggregate gradation and size affect the workability, layer thickness, rutting resistance, fatigue cracking, permeability, surface texture and frictional resistance.

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The properties of the bituminous mix including the density and stability are much dependent on the aggregates and their gra-

de sized distribution. Gradation has a profound effect on mix performance. It might be reasonable to believe that the best gradation is the one that produces maximum density. This would involve a particle arrangement where smaller particles are packed between larger particles, thus reducing the void space between particles. This creates more particle to particle contact, which would increase stability and reduce water infiltration of bituminous pavements. However, some minimum amount of void space is necessary to provide adequate volume for the binder to occupy, promote rapid drainage and provide resistance to frost action for base and sub base courses.

2. OBJECTIVE

- To study the effect of aggregate gradation and size on performance of cold bituminous mix
- To select suitable aggregate gradations within a specified gradation limit for cold bituminous mixes
- To evaluate several CBM (Cold Bituminous Mix) specimens in terms of Marshall Stability
- To evaluate the Optimum Emulsion Content (OEC) for different aggregate gradations.
- To evaluate the mechanical properties of the mix like Retained Marshall Stability.

3. MATERIALS USED

- Aggregate tests

Table.1 Properties of aggregates

Experiment	Test Results
Specific Gravity (CA)	2.74
Specific Gravity (FA)	2.67
Aggregate Impact value (%)	28.72
Aggregate Crushing value (%)	29.20
Los Angeles Abrasion Test (%)	28.43
Combined Elongation and Flakiness index (%)	25.53
Water absorption (CA) (%)	0.44

B. Bitumen Tests:

Table.2 Properties of bitumen emulsion

Experiment	Sample No	Test Results
Residue on 600 micron (%)	1	0.0175
	2	0.0425
	3	0.035
	Average	0.031
Coagulation at low temperature	1	Nil
	2	
	3	
Viscosity by SayboltFurol	1	236

Viscometer in Furol Seconds	2	252
	3	205
	Average	231
Storage stability after 24 hours (%)	1	0.96
	2	0.97
	3	0.97
	Average	0.96
Coating ability and water resistance a. Coating, dry aggregates b. coating, after water spraying	1	Good Fair
	2	
	3	
Residue by evaporation (%)	1	65.48
	2	66.21
	3	65.63
	Average	65.77
Tests on residue		
Penetration, mm	1	110
	2	89
	3	105
	Average	101.33
Ductility, cm	1	65
	2	68
	3	76
	Average	69.67

Specific gravity	1	1.01
	2	1.03
	3	1.01
	Average	1.016

Table. 4 Aggregate gradation for NMPS 19mm

C. Aggregate gradation:

Table.3 Aggregate gradation for NMPS 13.2mm

	MoRTH	Modi fied Fulle r andT homp son Grad ation	Gradati on based on Cooper 's et al. equatio n
Designati on	M13.2	MFT13.2	C13.2
Sieve size(m)	Percentageof passing (%)		
19	100	100	100
13.2	95	84.9	84.2
4.75	57.5	53.6	51.4
2.36	40	39.1	36.3
0.300	12.5	15.5	11.5
0.075	5.5	8.3	4

	MoRTH	Modi fied Fuller andT homp son Grada tion	Gradati on based on Cooper 's et al. equatio n
Designati on	M13.2	MFT13.2	C13.2
Sieve size(m)	Percentageof passing (%)		
26.5	100	100	100
19	95	86.1	85.6
9.5	70	63	61.8
4.75	50	46.1	44.3
2.36	35	33.7	31.4
0.300	11.5	13.3	10.4
0.075	5	7.1	4

4. EXPERIMENTAL WORK

A. Determination of Initial Emulsion Content (IEC)

$$IEC = 0.05A + 0.1B + 0.5C$$

Where, IEC = Initial emulsion content (%)
A = Percentage of aggregate retained on 2.36 mm sieve

B = Percentage of aggregate passing 2.36 mm sieve and retained on 75-micron sieve

C = Percentage of aggregate passing on 75-micron sieve

IEC for NMPS 13.2 mm is 9.2%.

B. Optimum pre-wetting water content

Pre-wetting of the aggregate is necessary because by adding the water initially to aggregate, it will become an optimum moist state so if bitumen emulsion added to the aggregate, it will not absorb the water content in the emulsion. If aggregate absorb the water from the emulsion, workability will reduce and very difficult to mix, and it won't provide the better coating. The coated aggregates are then visually observed for the coated area of the aggregate by the binder. The optimum water content is the water content at which the maximum coating of aggregate occurs. After carrying out several trials with water contents of 1, 2, 3, and 4%, the Optimum Pre-wetting Water Content was found to be 2%. This OPWC was adopted in this investigation for preparing cold bituminous mix specimens.

C. Marshall mix

The mixes were prepared according to the IRC: SP: 100-2014 cold mix design procedure.

The Marshall Stability and flow test is the performance prediction measure for the Marshall mix design method. The resistance of a compacted cylindrical sample of the bituminous mixture to plastic deformation is measured in this method when the sample is loaded diametrically at

a rate of 50 mm per minute. There are two significant features of the Marshall method of mix design. (i) density-voids analysis and (ii) stability-flow tests. The Marshall Stability of the mix is defined as the maximum load carried by the specimen at the specified standard test temperature. The flow value is the deformation undergone by the test specimen while loading up to the maximum load. The apparatus consists of the mould assembly, sample extractor, compaction pedestal, and hammer, breaking head, loading machine, flow meter, and water bath. Three compacted samples are prepared for each binder content in the Marshall mix design test method. To get the optimum binder content, at least five binder contents are to be tested. The following tests are carried out on all compacted specimens: determination of bulk density, stability and flow test and analysis of density and voids of the mix.

Table.5 Optimum pre-wetting water content for all gradations

	M13.2	MFT 13.2	C13.2	M19	MFT 19	C19
IEC (%)	9.5	10	8.5	9	9.5	8

O P W C (%)	2	2	1	2	2	1
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D. Retained Marshall Stability (RMS) Test

It is well known that the presence of moisture in a bituminous mix is a critical factor, which leads to premature failure of the flexible pavement. The loss of adhesion of aggregates with bitumen emulsion is studied by utilising Retained Stability Test to examine the effect of additive on resistance to moisture induced damage. Retained Marshall Stability is the ratio of soaked stability to dry stability. Both soaked stability and stability samples are prepared at OEC. After having been subjected to oven curing as explained for dry stability samples. The dry samples were water conditioned (capillary soaking). In this procedure, half the thickness of each compacted specimen is soaked in water at room temperature for 24 hours, the specimen is then inverted, and the other half was soaked for a further 24 hours. During soaking, the samples would rest on a bed of approximately 15 to 20 mm coarse sand. The samples are subsequently towel dried then tested for Marshall Stability at room temperature. The Marshall Stability test results obtained are referred to as Soaked Stability values.

$$\text{Retained Marshall Stability} = \left(\frac{\text{Soaked Stability}}{\text{Dry Stability}} \right) * 100$$

5. RESULTS & DISCUSSIONS

A. MARSHALL STABILITY TEST

The optimum composition of all six gradations was determined using the Marshall method for cold mix design as per the IRC:SP:100-2014. Marshall values for all gradations with different percentage of emulsion as shown in Below figures. The Marshall properties of such mixes with respect to the variation in residual bitumen content was studied. OEC for M13.2, MFT13.2, and C13.2 gradations were obtained as 8%, 9%, and 7% respectively. Also, OEC for M19, MFT19, and C19 gradations were obtained as 7.5%, 8%, and 7% respectively. OEC content decreased for an increase in the size for MoRTH and modified Fuller and Thompson gradations because the filler content is more in M13.2 and MFT13.2 than the M19 and MFT19 respectively and OEC remained same for Cooper's et al. gradations because of same filler content in the C13.2 and C19 gradations. High filler content have high surface area which requires more residual bitumen for coating. Marshall Stability values of M13.2 and M19 were found to be the highest among gradations under the categories of NMPS 13.2 mm and 19 mm respectively. The quantity of aggregate on NMPS sieve size is more for modified Fuller and Thompson and Cooper's et al. gradations than the MoRTH gradation. Among NMPS 13.2 mm gradations, Marshall Stability was found to be higher for MoRTH gradation, and it forms a dumbbell shape with an increase in bitumen emulsion. Bulk density increases and then decreases after reaching a peak value for MoRTH gradation and

continuously increases for other two gradations. Air voids fall in the range of 8 to 12 %, 10 to 12 % and 11 to 19 % for Fuller and Thompson, MoRTH and Cooper's gradations respectively. VMA decreases to lower peak and then increases and VFB increases for all gradations with the increase in bitumen emulsion as shown in fig 5.7. Among the NMPS 19 mm gradations, Marshall Stability was found to be higher for MoRTH gradation and varies rapidly with the increase in bitumen emulsion. These gradations also followed the same trend as NMPS 13.2 gradations in bulk density. Air voids fall in the range of 5 to 10 %, 11 to 14 % and 11 to 16 % for Fuller and Thompson, MoRTH and Cooper's gradations respectively as shown in fig 5.8. Marshall Stability values for C13.2 and C19 gradations showed a similar trend with closer values, but MoRTH and modified Fuller and Thompson not followed the same trend as size of aggregate increases as shown in figure below Bulk density increases and then decreases after reaching a peak value for M13.2 and M19 gradations, but increases for all other gradations with an increase of RBC shown in fig 5.9(b). Air voids decreases and flow value increases for all gradations with an increase in RBC as shown in fig 5.9(c) and 2(d). Air voids fall in the range of 5 to 14 %, 11 to 19 % and 10 to 14 % for Fuller and Thompson, Cooper's and MoRTH gradations respectively. It was observed that Marshall Stability decreases and VMA increases with increase in aggregate size for MoRTH gradation.

Gradation	Optimum Bitumen emulsion (%)	Optimum Residual Bitumen (%)	Marshall Stability (kN)	Flow Value (mm)	Bulk Density (g/cm ³)	Air Voids (%)	VMA (%)	VFB (%)
M13.2	8	5.23	8.13	4.19	2.214	10.62	11.46	22.08
MFT 13.2	9	5.88	7.06	5.00	2.211	9.79	12.87	22.66
C13.2	7	4.57	6.95	4.85	2.121	15.96	9.24	38.86
M19	7.5	5.55	7.44	5.63	2.180	11.67	11.98	23.65
MFT 19	8	5.23	7.32	5.25	2.271	8.28	11.76	20.04
C19	7	4.57	7.15	4.97	2.153	14.92	9.74	23.76

Table.6 Marshall Stability test results for all gradations at ORBC

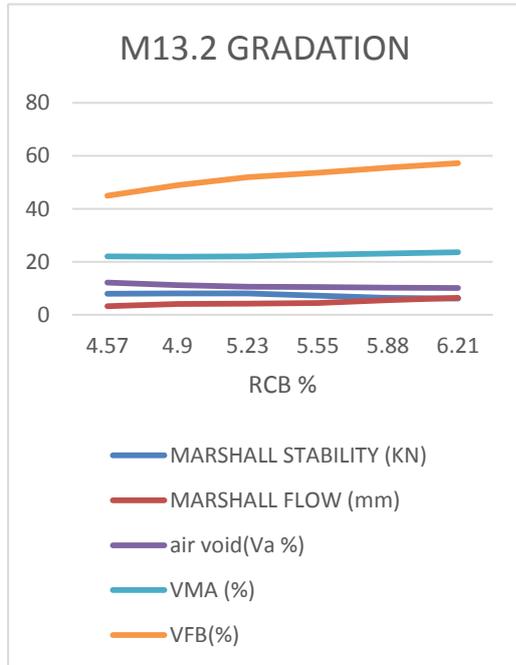


Fig.1 Marshall graphs for M13.2 gradation

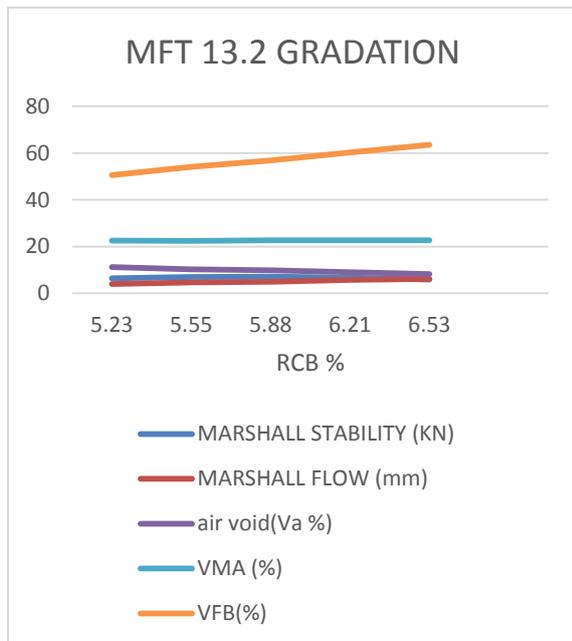


Fig .2 Marshall graphs for MFT13.2 gradation

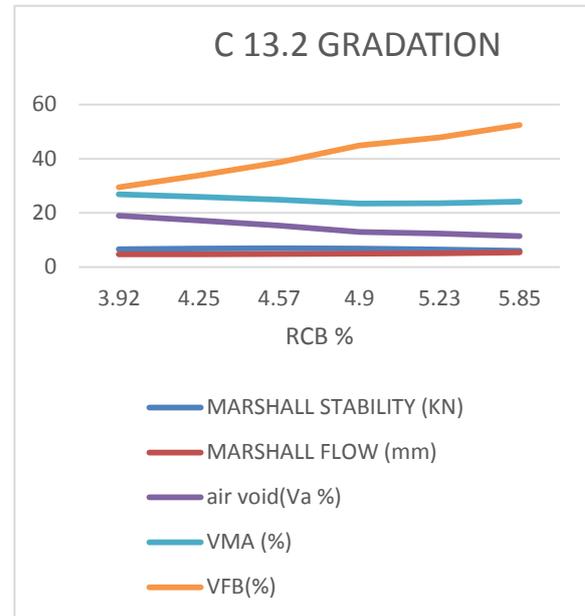


Fig .3 Marshall graphs for C13.2 gradation

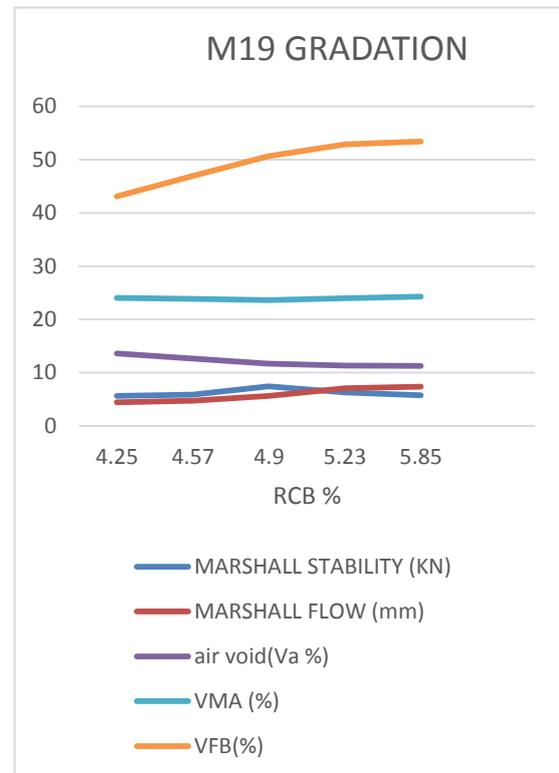


Fig .4 Marshall graphs for M19 gradation

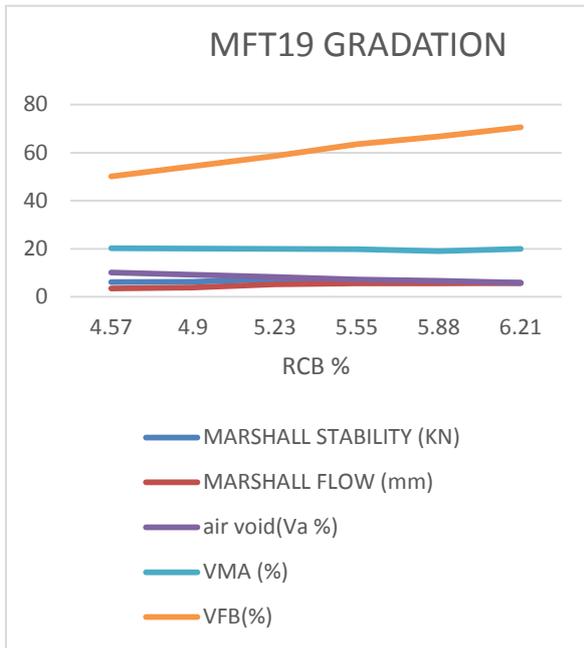


Fig .5 Marshall graphs for MFT19 gradation

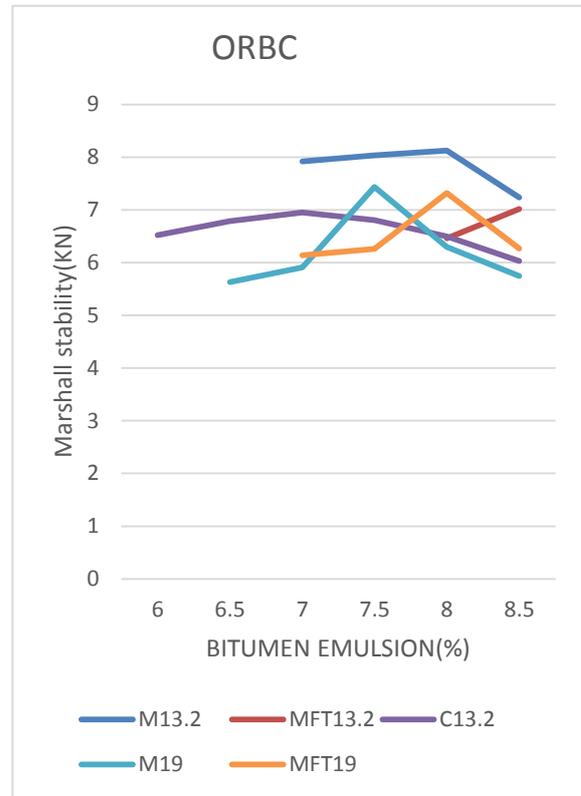


Fig .7 Marshall stability for various gradation

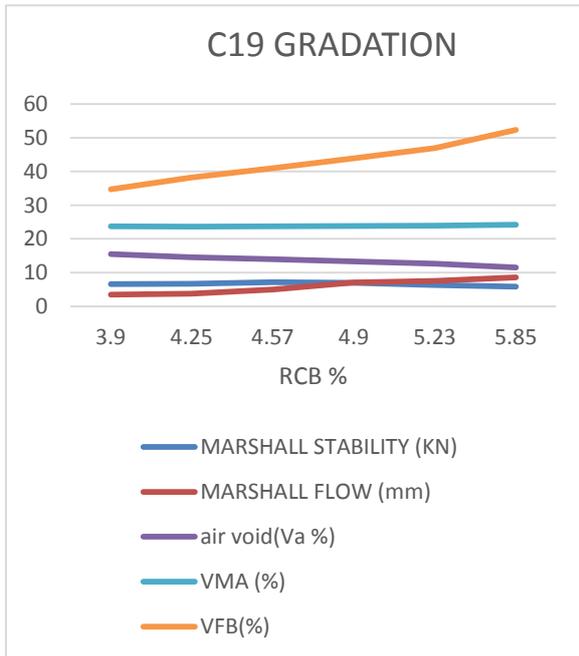


Fig..6 Marshall graphs for C19 gradation

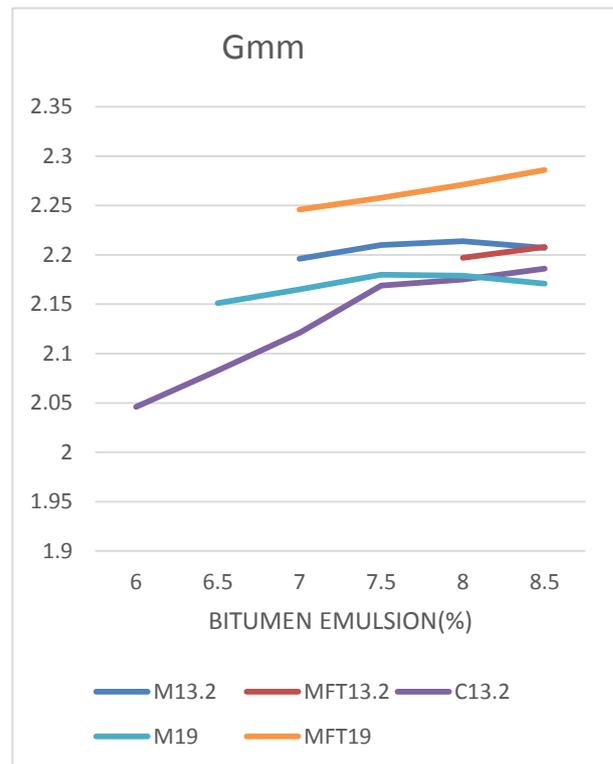


Fig .8 Bulk specific gravity (Gmm) for various gradation

B. RETAINED MARSHALL STABILITY

Soaked stability test conducted after soaking half thickness of the sample in water for one day and reversed for one day. Soaked stability values was found to be higher than the dry stability values because of the possible reaction of emulsifiers with water. Retained Marshall Stability was calculated for all gradation at OEC. RMS was found to be higher than the 100% for all gradations and the results are shown in below table.

Gradations	OEC	Dry Marshall stability	Soaked Marshall stability	Retained Marshall Stability
	%	KN	KN	%
M13.2	8	8.13	9.23	113.53
MFT13.2	9	7.06	7.15	101.27
C13.2	7	6.95	7.88	113.38
M19	7.5	7.44	8.11	109.01
MFT19	8	7.32	8.19	111.89

C19	7	7.15	9.15	127.97
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Table .7 Retained Marshall Stability for all gradations

6. SUMMARY

Two aggregate gradations were taken based on the NMPS: NMPS 13.2 mm and NMPS 19 mm from Ministry of Road Transport and Highways (MoRTH) specification. Further, two more aggregate gradations were formulated for each NMPS based on modified Fuller and Thompson maximum density gradation and Cooper's et al. equation. The study methodology included characterization of aggregates and emulsion, determination of pre-wetting water content, volumetric analysis of mixes by Marshall Stability test, performance evaluation by Retained Marshall Stability (RMS). A parameter called gradation ratio was correlated with strength and performance parameters of the mix to predict later from aggregate gradation curve alone.

7. CONCLUSION

- a. Aggregate gradation affects the Marshall design parameters. The quantity of coarse aggregate equal to NMPS sieve size is more in the modified Fuller and Thompson and Cooper's et al. gradation compare to MoRTH gradation. It leads to less

- interlock by the presence of filler quantity. For, both NMPS 13.2 mm and 19 mm categories, MoRTH gradations gave the peak stability values.
- b. Marshall Stability value increases with the increase in the size of the aggregate for modified Fuller and Thompson gradation and Cooper's gradation but decreases with increase in the size of aggregate for MoRTH gradation.
 - c. For MoRTH gradations, as NMPS increases VMA increases, but for Fuller and Thompson and Cooper's gradations as NMPS increases VMA decreases.
 - d. From the above two points, it can be concluded that as the size of the aggregate increases, Marshall Stability increases and VMA decreases, and vice versa.
 - e. As the size of the aggregate in a particular gradation increases, optimum residual bitumen content decreases for MoRTH and modified Fuller and Thompson gradation, but remains same for Cooper's et al. gradation. This is because, the filler content in NMPS 13.2 mm gradation of MoRTH and modified Fuller and Thompson is higher than NMPS 19 mm gradations, and filler content is same for Cooper's gradation. Surface area is high for higher filler quantity. Higher surface area requires high emulsion content for full coating.
 - f. Air voids also changes with change in gradation and NMPS. Air voids for MFT 13.2 is 9.79 and for MFT 19 is 8.28% at OEC. These values are less compared to other gradation because of higher filler quantity.
 - g. Cooper's gradation shows better rutting resistance among three gradations of NMPS 13.2 mm and NMPS 19 mm.
 - h. Retained Marshall Stability is above the 100% for all gradations.

8. FUTURE STUDY

- a. The same study can be conducted with different filler variations in the Cooper's et al. equation.
- b. Comparison of CBMs with Hot Mix Asphalt (HMA) can be studied by conducting the performance tests with all the six gradations using HMA.
- c. Study with CBMs using RAP material can be conducted to save the material and save money.
- d. The study can be extended with different curing temperatures and curing period.
- e. Wheel tracking test can be performed with & without the presence of water.
- f. The other mix performance studies can be conducted to understand the fatigue behaviour of the mixture.

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